# **Assessment Report**

# Margaret Creek Watershed Margaret Creek 6, Fox Lake Athens County, Ohio

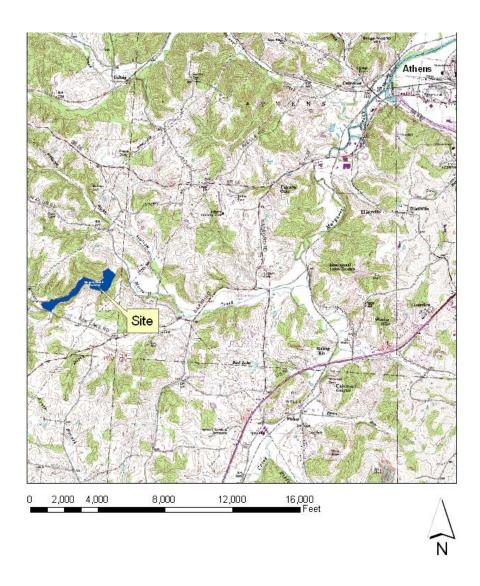


Prepared by: Natural Resources Conservation Service Columbus, Ohio September 2009

## **Table of Contents**

Project Location	1
Description of Margaret Creek 6	2
Sponsors of Margaret Creek Watershed	2
Plan View	3
Brief History and Existing Condition	3
Hazard Classification	4
Status of Operation and Maintenance	4
Rehabilitation Needs	4
Eligibility for Dam Rehabilitation Program	5
Potential for Addressing Other Resource Needs	5
Potential Scope of the Rehabilitation Project	5
Rehabilitation Process	6
Breach Analysis	7
Attachment A – Breach Inundation Mapping	8
Attachment B - Evaluation of Potential Rehabilitation Projects spreadsheet	12

**Project Location**Margaret Creek 6 is located approximately 4 miles southwest of Athens, south of the intersection of Township Road 29 and Township Road 27.



### **Description of Margaret Creek 6 (Fox Lake)**

Fox Lake is Site 6 for the Margaret Creek Conservancy District (MCCD), which is a subdistrict of Hocking Conservancy District (HCD). Margaret Creek 6 is a multiple purpose lake that provides flood control and recreation opportunities. MCCD cooperates with ODNR, Division of Fish/Wildlife in operating the lake. Margaret Creek 6 is a compacted earth fill dam that is 46 feet high, 610 feet long, with 3 horizontal to 1 vertical (3 to 1) upstream side slope, and 2.5 to 1 downstream side slope. There is a 10-foot wide wave berm located at the permanent pool elevation on the pool side of the embankment. The drainage area is 2,566 acres (4.0 square miles). A permanent pool of 47.5 surface acres provides 465 acre-feet of storage (160 acre-feet for 100-years of sediment storage and 305 acre-feet for recreation). The principal spillway is a reinforced concrete pipe system, which maintains the normal pool level and regulates the passage of flood flows. It consists of an NRCS standard covered top riser 26 feet high, a 30-inch diameter reinforced concrete pipe, and a stilling basin/plunge pool as an outlet structure. The emergency spillway is 40 feet wide and is designed to safely pass 6.75 inches of rainfall occurring in a 6-hour period. The dam was also designed to pass 13.0 inches of rain occurring in a 6-hour period without overtopping the dam. A 12-inch diameter lake drain allows the lake to be lowered for maintenance. The NID reference for Margaret Creek 6 is OH00706.

During flood events, the dam was designed to store 710 acre-feet of floodwater up to the auxiliary spillway elevation which would be slowly released through the principal spillway. There is 8 feet of elevation between the auxiliary spillway and top of dam. Total storage at top of dam is 2060 acre-feet.

Potential seepage along the pipe system is controlled with six 9' x 13' concrete anti-seep collars surrounding the concrete principal spillway pipe. There is a foundation trench drainage system along the downstream toe with 6 inch corrugated metal pipes.

#### **Sponsors of Margaret Creek Watershed**

The Sponsors of the Margaret Creek Watershed project include Margaret Creek Conservancy District, the Ohio Department of Natural Resources, the Athens Soil and Water Conservation District, Athens County Commissioners, and the Village of Albany.

#### Plan View



#### Margaret Creek 6 Dam

### **Brief History and Existing Condition**

The original Watershed Work Plan for the Margaret Creek Watershed was developed by the Natural Resources Conservation Service (then Soil Conservation Service in 1966. Margaret Creek 6 is one of five floodwater-retarding structures built within the Margaret Creek watershed from 1967 to 1972 under the authority of the Watershed Protection and Flood Prevention Act (Public Law 566, 83<sup>rd</sup> Congress). Margaret Creek 6 was constructed in 1967.

The structure was planned and built with the primary purpose of flood control. It was designed to have a 100 year economic life. It is used for recreation activities including boating and fishing. The structure is in overall good condition. In the fall of 2007 flow from the right foundation drain (looking downstream) increased from a trickle to pencil size. This clear water flow has continued with some fluctuation with pool level. The MCCD continues to closely monitor the flow rate. According to the MCCD the auxiliary spillway has not experienced flow.

#### **Hazard Classification**

Margaret Creek 6 was originally planned and designed as a significant hazard structure since it primarily protected agricultural lands. Residential development was not anticipated, and there was no anticipated loss of life in the event of a dam failure.

The ODNR, Division of Water, Dam Safety Engineering, has regulatory responsibility for dam safety in Ohio. Margaret Creek 6 was originally reclassified as a high hazard structure by ODNR in 1983. ODNR Dam Safety reclassified this structure as Class I (high hazard) based on visual observations and potential hazard downstream of the dam. ODNR Dam Safety and NRCS criteria require high hazard structures to safely pass 100% of the Probable Maximum Flood (PMF). A breach analysis was completed by NRCS in 1994 that confirmed high hazard classification. In 2001, ODNR Dam Safety performed hydrologic and hydraulic calculations to estimate the capacity of the dam and the size of the design flood. These calculations indicate the dam can only pass 40% of the design flood (PMF). An Emergency Action Plan (EAP), with breach inundation map, was completed in 2003. A copy of the hazard class documentation, breach analysis summary, and revised breach inundation map are included in Attachment A of this report.

#### **Status of Operation & Maintenance**

Margaret Creek Conservancy District has a current Operation and Maintenance (O&M) agreement with NRCS to perform O&M for the structure. The O&M agreement expires on May 13, 2067. The remaining evaluated life of the structure is 58 years. The conservancy district performs the required annual inspections every April or May. NRCS has assisted with these annual inspections.

Past and current ODNR dam safety reports indicate the dam is in good condition and has been well maintained. There are hairline cracks in the riser and concrete cradle that need to be monitored. ODNR Dam Safety has formally inspected the dam in 1974, 1983, 1992, 2001, and 2007. According to these inspection records, the owner must develop "plans and specifications as necessary to increase the discharge/storage capacity to pass the required design flood". The minimum design flood for Class I dams is 100% of the Probable Maximum Flood

#### **Rehabilitation Needs**

Several items need to be addressed in order for Margaret Creek 6 to meet current State Dam Safety and NRCS criteria associated with a high hazard structure. The rehabilitation program requires that the useful life of the structure must be extended beyond the original evaluated life. The evaluated life for Margaret Creek 6 must extend past the year 2067. General rehabilitation work would include:

- Modify the dam and auxiliary spillway to safely pass or contain the larger runoff from the rainfall required for design of a high hazard structure. This may consist of raising the dam and/or widening the emergency spillway, adding a roller compacted concrete (RCC) chute spillway through the dam, or combinations of these.
- 2. Ensure that appurtenant structures (riser tower, internal drains, etc.) meet current NRCS and State Dam Safety criteria.

3. Ensure that the sediment pool has a minimum sediment storage capacity that matches the rehabilitated evaluation life period.

### **Eligibility for Dam Rehabilitation Program**

Margaret Creek 6 is eligible for NRCS assistance authorized under the Rehabilitation provisions of the Small Watershed Program. Funding for rehabilitation is based upon a priority ranking system, which considers the potential for dam failure and the potential consequences of dam failure. High hazard structures are given a higher ranking for funding than low hazard structures. A completed Evaluation of Potential Rehabilitation Projects spreadsheet is included in Attachment B of this report.

The Sponsors of the potential rehabilitation project should be aware that additional landrights might be required for construction. The Sponsors are responsible for paying this cost but this cost can be included in the total project cost of the rehabilitation project.

The rehabilitation provisions of the PL 106-472 can provide 65% of the total rehabilitation cost, but shall not exceed 100% of the actual construction costs incurred in the rehabilitation. Total rehabilitation cost for the project shall include all costs associated with all components of the project, including acquisition of land, easements, rights-of-way, project administration, non-Federal technical assistance (TA), non-structural measures, contracting, and construction. The cost of TA provided by NRCS shall not be considered part of the total cost of the rehabilitation project. If the Sponsors provide or otherwise obtain TA for planning, design, and/or construction, the TA cost is included in the computation of total cost of the rehabilitation project. The Sponsor is responsible for the cost of all water, mineral, and other resource rights and all federal, state, and local permits, which are not considered part of the total cost of the rehabilitation project. The Sponsors' 35% can be in the form of cash, in-kind services, the value of land rights in addition to those acquired for the current project, or any combination of these items.

#### **Potential for Addressing Other Resource Needs**

If rehabilitation is pursued, the Sponsors will have the opportunity to investigate the addition of other purposes to the site. There are no known additional resource needs at this time.

#### Potential Scope of the Rehabilitation Project

The following are potential rehabilitation alternatives that exist for the site.

- 1. Rehabilitate the structure to meet current State Dam Safety and NRCS criteria for a high hazard structure. The structure must be able to safely pass the PMF. These options briefly outlined below:
  - Alternative 1 Widen the spillway from 40 feet to 330 feet without modification to the dam. Approximately 375,000 CY of excavation would be required. A splitter dike would be required in the spillway to divide the flow. The cost estimate range is \$3,500,000 to \$4,000,000.

- Alternative 2 Raise the top of dam 3 feet and widen the spillway from 40 feet to 150 feet. Approximately 150,000 CY of excavation and 15,000 CY of earthfill would be required. It is assumed that the excavated material could be used to raise the dam. The cost estimate range is \$2,000,000 to \$2,500,000.
- Alternative 3 Similar to Alt 2 above except excavation and earthfill quantities would be balanced. The top of dam would be raised 7.4 feet and the spillway widened from 40 feet to 65 feet. Approximately 40,000 CY of excavation and earthfill would be required. The auxiliary spillway would need to be armored to be stabile during high flow. The 30 inch pipe would need to be extended approximately 40 feet and the plunge pool would be replaced with an impact basin. The cost estimate is \$1,250,000 to \$1,750,000.
- Alternative 4 Construct a 200-foot wide roller compacted concrete (RCC) chute spillway through the dam to increase spillway capacity. Dam and existing auxiliary spillway would remain as they are currently. Cost estimate is \$1,000,000 to 1,250,000.
- Another alternative considered included raising the dam 8.7 feet without
  widening the spillway. The existing 40-ft wide spillway would however need
  to be armored to remain stable. The 30 inch pipe would need to be extended
  almost 50 feet and the plunge pool would be replaced with an impact basin.
  No cost estimate was calculated for this option.
- 2. Remove the downstream hazards and enact zoning restrictions within the breach inundation zone to prevent future development. This option is not considered viable due to the extent of development and the low likelihood of additional local zoning being enacted that would restrict future development.
- 3. Remove or breach the structure to eliminate the capacity of the structure to retain floodwater. This would eliminate the potential for a breach of the structure during a storm event. Since the O&M agreement with NRCS has not expired, this option may require the sponsor to reimburse the federal government for any remaining benefits that the structure may provide over the remainder of the lifespan of the O&M agreement. This option is not considered viable due to the local reliance on the flood control benefits provided by the structure.

#### **Rehabilitation Process**

The Sponsors submitted an application for federal dam rehabilitation assistance on March 16, 2007. The application included all of the required items.

If the project is selected for planning, the site will go through the conventional watershed planning process with consideration and evaluation of all potential alternatives and their impacts (economically, environmentally, socially, etc.). During the planning process, there will be opportunities for public participation and comment.

The estimated time frames for the activities are:

Planning: 1 year minimum

Design: 1 yearImplementation: 1 year

### **Breach Analysis**

NRCS conducted a breach analysis and evaluated the hazard classification for Margaret Creek 6. The dam is located in Section 1, Waterloo Township, Athens County, Ohio, on the West Branch Margaret Creek, approximately 2.3 miles upstream of the confluence with Margaret Creek. The analysis continued downstream approximately 2.7 miles to the Hocking River in the City of Athens.

To evaluate the hazard classification, NRCS performed a sunny day breach analysis with the water level at the crest elevation of the auxiliary spillway. First, the minimum peak discharge for a breach of this dam was calculated based on the criteria in NRCS *Technical Release 60, Earth Dams and Reservoirs*. The minimum peak discharge was then used to calculate the breach hydrograph using criteria in NRCS *Technical Release 66, Simplified Dam breach Routing Procedure*. Flood discharges expected downstream were determined by routing the breach hydrograph through valley cross sections downstream of the dam, using NRCS WinTR-20 Program. The peak discharges downstream were input into HEC-RAS (USACE) to determine water elevations. The breach evaluation extended downstream to the point where the "sunny day" breach flood depth equals the 100-year flood depth without a breach.

The results of the breach analysis are shown on the dam breach inundation maps (attached). Based on the latest available ortho imagery (2007), there are 5 bridges, 10 unclassified structures (homes, businesses, outbuildings), and 40+ house trailers within the breach inundation area. There is the potential for loss of life in the event of a dam failure.

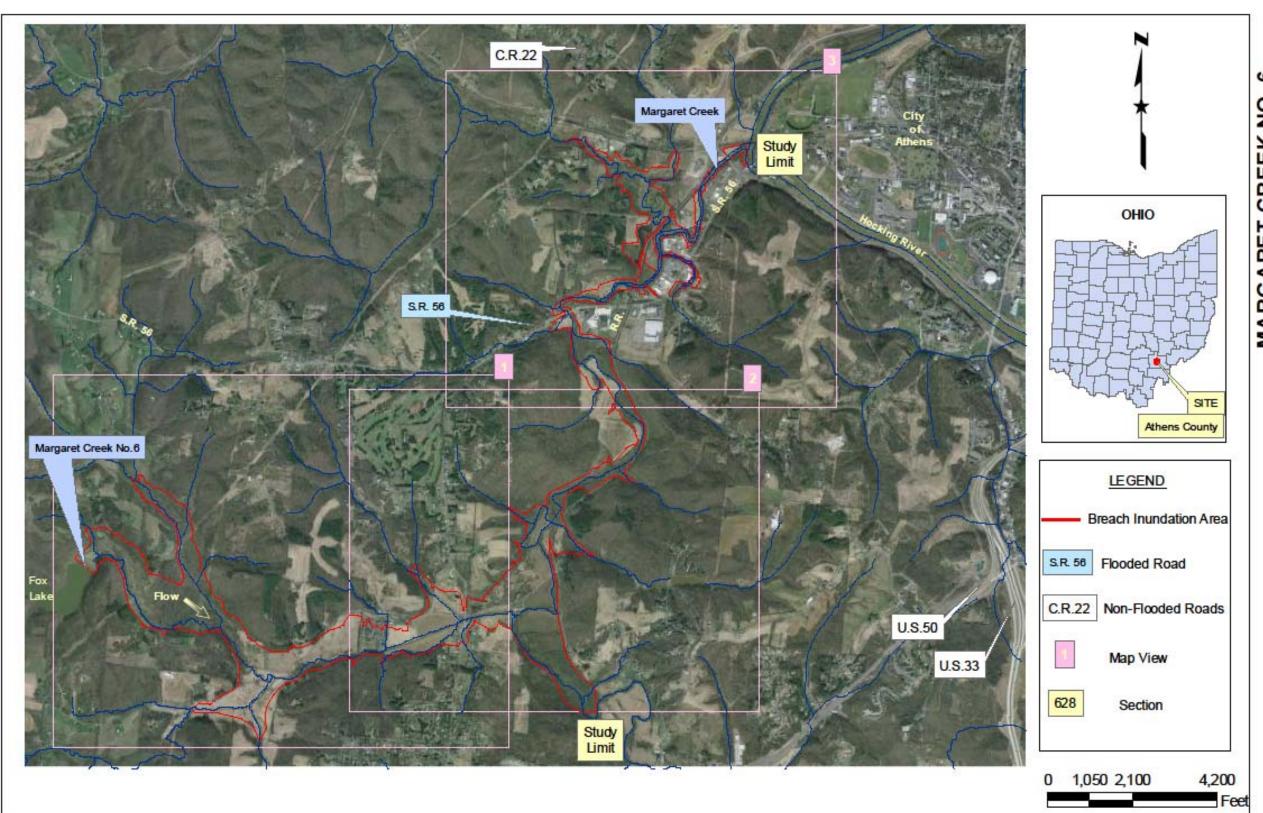
Based on this analysis, the NRCS has confirmed the classification of Margaret Creek 6 as a high hazard dam.

Scott J. Jerrome Planning Engineer

Scott & Jerron

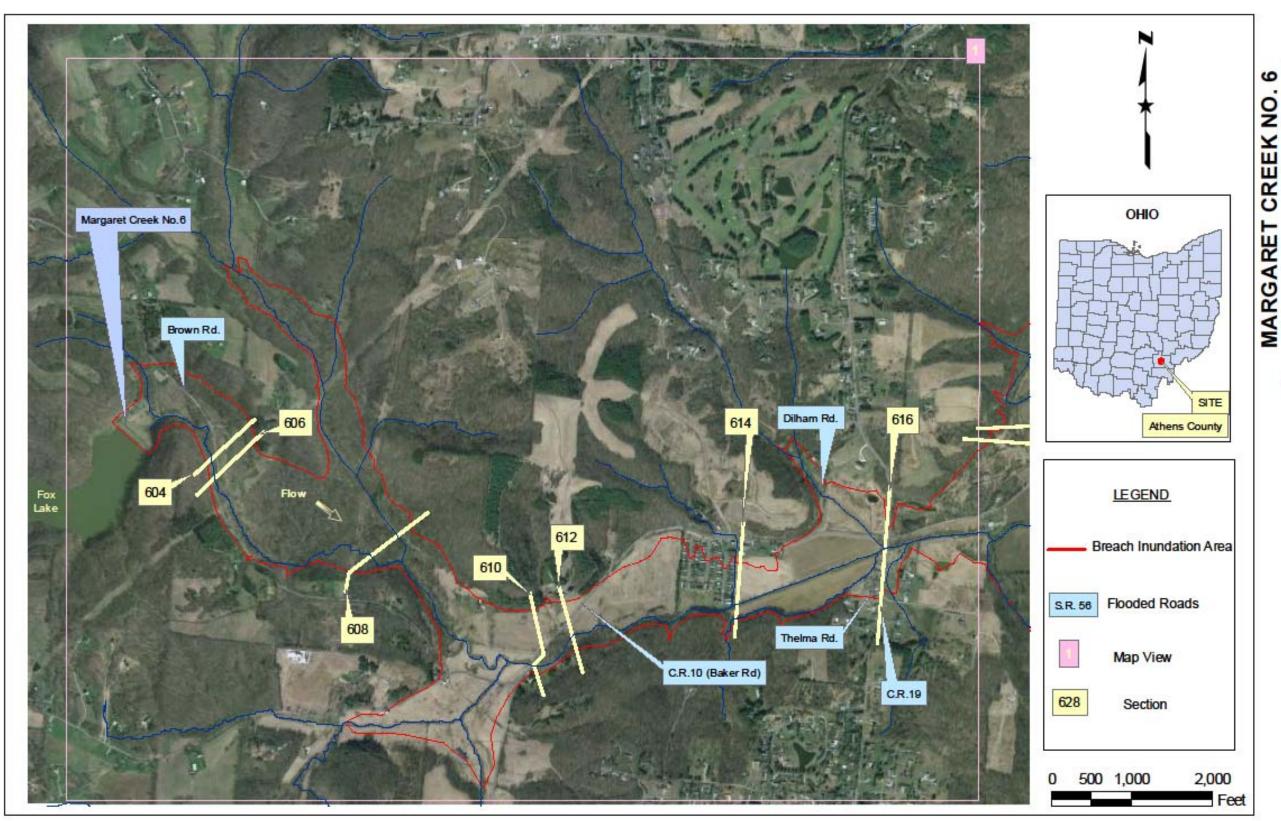
Michael J. Monnin, P.E. State Conservation Engineer

Michael & Monnin



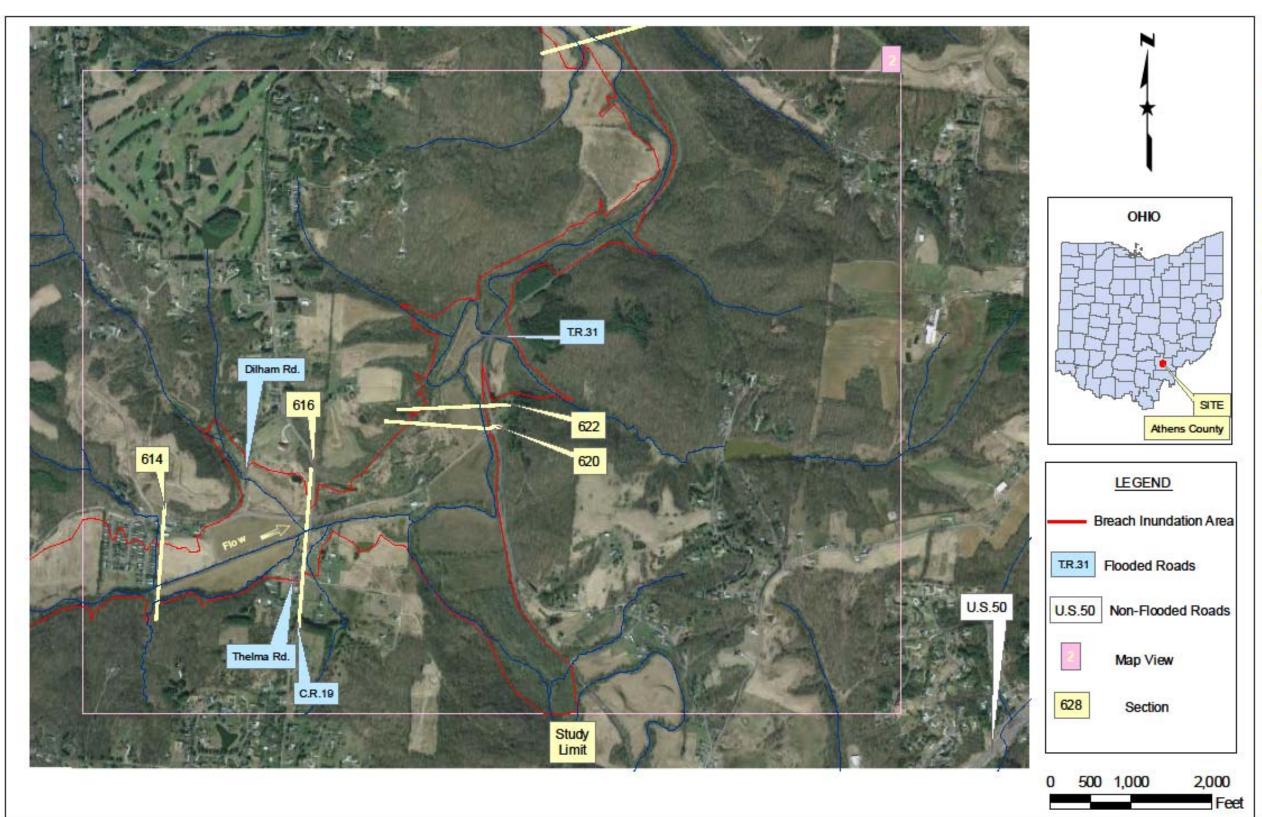
MARGARET CREEK NO. 6

Dam Breach Inundation Boundary
NDEX SHEET - (3) MAPS
Athens County, Ohio 8009



Dam Breach Inundation Boundary
MAP 1
Athens County, Ohio

Note: The Breach Inundation Area is the area expected to flood if Margaret Creek Dam No. 6 on Fox Lake failured suddenly when the water elevation is at the auxiliary spillway crest. Limits of flooding may vary from actual locations on the ground due to photographic displacement.

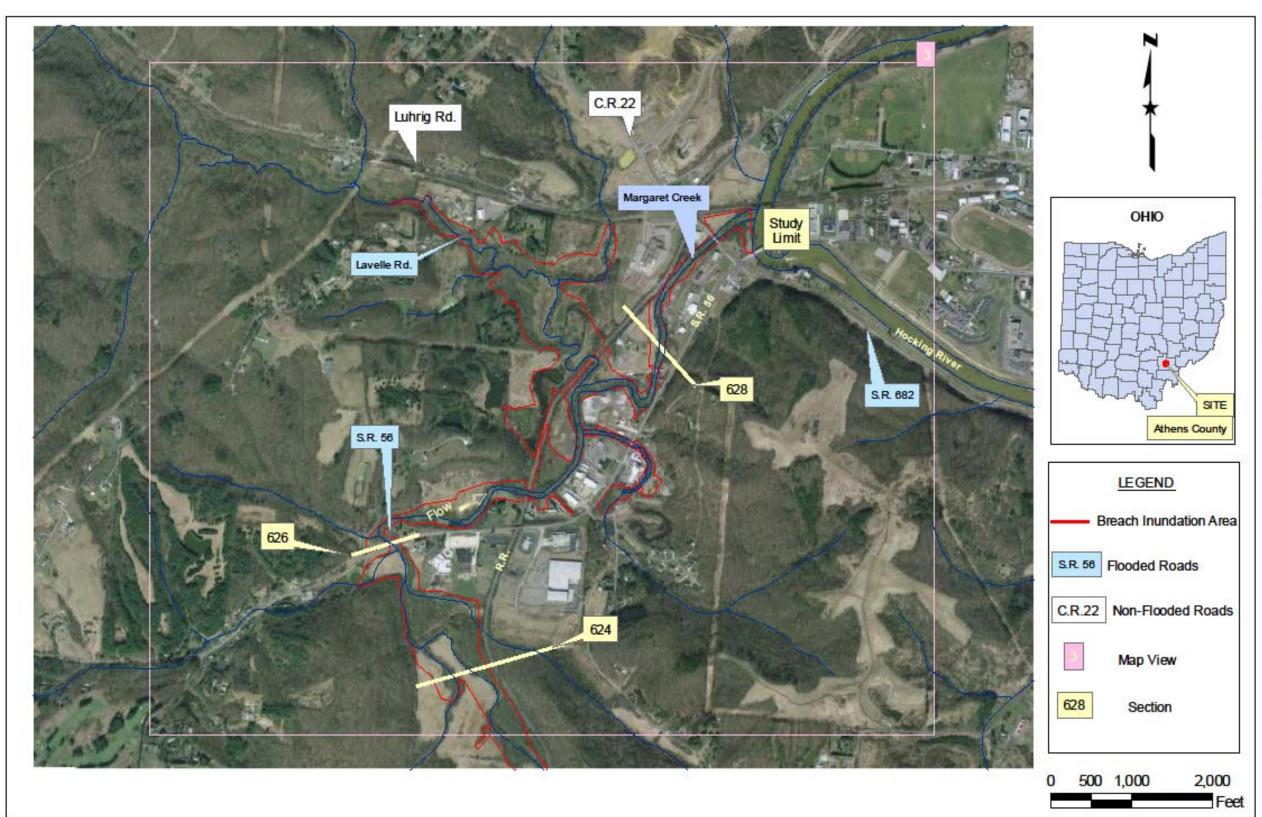


MARGARET CREEK NO. 6

Dam Breach Inundation Boundary
MAP 2

Athens County, Ohio 800

Note: The Breach Inundation Area is the area expected to flood if Margaret Creek Dam No. 6 on Fox Lake failured suddenly when the water elevation is at the auxiliary spillway crest. Limits of flooding may vary from actual locations on the ground due to photographic displacement.



MARGARET CREEK NO. 6

Dam Breach Inundation Boundary
MAP 3

Athens County, Ohio 809

1	- I	EVALUATI	ON OF POTENTIAL REHABILITATI	ON PROJEC	TS		
STATE OH DA	AM M	argaret Cre	eek Str 6	BY SJJ	DATE	8/19/	09
YEAR E	BUILT	1967	DESIGN HAZARD CLASS	M DRA	INAGE AREA	4	mi2
WORK PLAN	DATE	1966	CURRENT HAZARD CLASS	H	DAM HEIGHT	42	ft
sht 1 of 5		C	ONSEQUENCES OF DAM FAILURE			ver 1001	01
POTENTIAL DAM	FAILUR	E:			109		-
Total Failure Inde	×					199	Α
POTENTIAL LOSS	OF LIF	E:			0.00	11:	
Maximum Popula	tion-at-F	Risk [PAR]			(number)	125	В
Total Risk Index					- 0	746	С
POTENTIAL LOSS	OF PR	OPERTY:					
1000 TO 1000 TO 1000	100	affected by	breach and rate impact as High (H),	Medium (M),	Low (L) or No	_	_
Community At			3 S W S O S O S O S O S O S O S O S O S O		(H,M,L,-)	Н	D
Number of hom					(number)	50	E
POTENTIAL LIFEL							
		nmunity dis	rupted by dam failure, and estimate r		200		-
Municipal sole :				Users	(number)	0	F
Supplemental s	_			Users	(number)	0	G
Irrigation water	CONTRACTOR OF THE PARTY OF THE	aranay kuta	THE STATE OF THE S	Storage	(Ac-Ft)	0	Н
POTENTIAL INFRA		CONTRACTOR OF STREET					
			entify major crossing rendered unusal	STATE OF THE PARTY			
Major/Interstate		t Rt 56		Roads	(number)	1	- 1
			. 10, C.R. 19, T.R. 31	Roads	(number)	4	J
			THE ENVIRONMENT:				
The second secon			ligh (H), Medium (M), Low (L), or Nor	ne (blank)		110	1
Threatened & e	200				(H,M,L,-)	L	K
Sensitive riparia	W. 10 C. 10				(H,M,L,-)	L	L
Contaminated r					(H,M,L,-)	L	M
Wetland and wi	ildlife hal	bitat			(H,M,L,-)	L	N
Other					(H,M,L,-)	Š.	0
POTENTIAL ADVE							
			ligh (H), Medium (M), Low (L) or Non	e(blank)			-
Known cultural		10 m	none known		(H,M,L,-)	L	P
Historic preserv			none known		(H,M,L,-)	L	Q
Socially disadva	_				(H,M,L,-)	L	R
POTENTIAL ADVE					184		-
A CONTRACT OF THE PARTY OF THE			this dam, updated workplan value		(\$)	(5)	S
		worknlan-	Increase(I), No change(NC), Decrea	ise(D)	(I,NC,D)	1	Т
Low income famil	lies impa						
		cted			(number)		U
		acted AFETY AG	10 March 1987 1987 1988 1988 1988 1988 1988 1988		900		
State dam safety	order is	acted AFETY AGI sued for rep	pair, modification, removal issued, Ye		(Y,N)	Υ	] v
State Dam Safety	order iss Agency	acted AFETY AG sued for rep Priority, H	10 March 1987 1987 1988 1988 1988 1988 1988 1988		900	Y	
State dam safety State Dam Safety OTHER CONSIDE	order iss Agency RATION	acted AFETY AGI sued for rep y Priority, H IS:	pair, modification, removal issued, Yeigh(H), Medium(M), Low(L), None(bla	ank)	(Y,N) (H,M,L,-)		
State dam safety State Dam Safety OTHER CONSIDE Identify any other	order iss Agency RATION conside	acted AFETY AGI sued for rep y Priority, H IS:	pair, modification, removal issued, Ye	ank)	( Y,N ) (H,M,L,-)		· w
State dam safety State Dam Safety OTHER CONSIDE	order iss Agency RATION conside	acted AFETY AGI sued for rep y Priority, H IS:	pair, modification, removal issued, Yeigh(H), Medium(M), Low(L), None(bla	ank)	(Y,N) (H,M,L,-)		- 1

EVALUATION OF POTENTIAL REHABILITATION PROJECTS									
STATE	OH DAM	Margaret Creek Str 6	BY	SJJ	DATE	8/20/09			
sht 2 of 5	prott Keneralitation	FAILURE & RISK INDE	XES	. pa. 2241 6	APAILESI O.	ver 102201			

Adopted from Bureau of Reclamation "Risk Based Profile System"

see: http://www.usbr.gov/dsis/risk/rbpsdocumentation.pdf

#### LIFE LOSS:

Population-at-Risk [PAR], see NRCS dams inventory definition (number of people)

Estimate PAR for static loading failure, typically assume water at top of dam

Estimate PAR for hydrologic loading failure, typically assume water at top of dam

Estimate PAR for seismic loading failure, typically assume water at ES crest (sunny day failure)

125 A 125 B 100 C

Fatality Rates [FR] from dam breach

Adopted from BuRec "A Procedure for Estimating Loss of Life Caused by Dam Failure" DSO-99-06

see: http://www.usbr.gov/research/dam\_safety/documents/dso-99-06.pdf

Flood Severity/Lethality [DV] is the average depth [D] times velocity [V] across flood plain (ft2/sec)

DV= (breach discharge - bank full discharge) / breach floodplain width

Warning Time [T] between failure warning and flood wave at population (minutes)

Flood Severity Understanding [U] of the warning issuer of the likely flooding magnitude

scenario	breach discharge	bankfull discharge	breach width	DV	warning time	under- standing
H-100 EC-190300	(cfs)	(cfs)	(ft)	(ft2/sec)	(minutes)	(N/A or Vague)
Static	54100	120	550	98	60	vague
Hydrologic	54100	120	550	98	60	vague
Seismic	21700	120	500	43	60	vague

For	DV>50	T=0	U=N/A (no warning)	FR=0.15
For	DV>50	T<80	U=vague	FR=0.04
For	DV>50	T>60	U=vague	FR=0.03
For	DV<50	T=0	U=N/A (no warning)	FR=0.01
For	DV<50	T<60	U=vague	FR=0.007
For	DV<50	T>60	U=vague	FR=0.0003

Estimate FR for static loading failure scenario

Estimate FR for hydrologic loading failure scenario

Estimate FR for seismic loading failure scenario

0.03 D 0.03 E 0.007 F

Scenario	Load Factor	Response Factor	Failure Index	Fatality Rate	PAR	Risk Index
Static	1	126	126	0.03	125	473
Hydrologic		*	73	0.03	125	274
Seismic	0.00	#DIV/0!	0	0.007	100	0
117		TOTAL=	199		TOTAL=	746

	EVALUATION OF POTENTIAL REHABILITATION PROJECTS		
STATE	OH DAM Margaret Creek Str 6 BY SJJ DATE	8/19/	09
sht 3 of 5	STATIC FAILURE INDEX	ver 100	101
PRINCIPAL	SPILLWAY SYSTEM (60 points max): (total points) 30		Α
Downstre	am filter or filter zone around conduit (yes=0 or no=10)	10	В
Conduit to	rench deep (>2d) and narrow (<3d) and steep sideslope (<2:1) (no=0 or yes=10)	0	C
Principal	spillway system (inlet, pipe, or outlet) in deteriorated condition (no=0 or yes=10)	0	D
Conduit h	as seepage cutoff collars or other compaction adverse features (no=0 or yes=10)	10	Ε
Conduit o	ontains open joints, open cracks, steady seepage (no=0 or yes=10)	0	F
Conduit fo	ounded on competent bedrock (yes=0 or no=10)	10	G
Reservoir	control gate located at outlet of conduit (no=0 or yes=10)	0	Н
RESERVO	R FILLING HISTORY (75 points max): (total points) 10		. 1
Reservoir	has filled to x% of effective height (earth spillway crest minus original streambed)	91	J
(<50%=7	5 or 51-75%=50 or 76-90%=25 or 91-95%=10 or 96-100%=5 or >100%=0)	10	K
SEEPAGE	AND DEFORMATION (85 points max): (total points) 80		L
Seepage	carrying fines, or seepage increases with reservoir elevation increases, or	<u> </u>	_
	sinkholes/jugholes exist in embankment (no=0 or yes=80)	80	M
Large am	ounts of seepage (no=0 or yes=6)		N
Visible an	d significant slope movement or sloughing (no=0 or yes=6)	0	0
Longitudi	nal or transverse embankment cracking greater than one foot in depth (no=0 or yes=6)	0	P
Sinkholes	/depressions within two times effective height of the dam, either face (no=0 or yes=6)	0	Q
Poor top	of dam condition, eroded, trees, rodent holes, settlement (no=0 or yes=8)	0	R
Abnorma	ly wet areas at downstream toe/groin of embankment (no=0 or yes=8)		S
Inadequa	te slope protection against erosion by rainfall or waves (no=0 or yes=6)	0	Т
FOUNDAT	ION GEOLOGY (41 points max): (total points)		U
Highly fra	ctures rock under core (no=0 or treated=3 or untreated=30)	0	V
Karst terr	ain and soluble rock (gypsum or limestone) (no=0 or treated=3 or untreated=30)	0	W
Collapsib	le soils (no=0 or treated=3 or untreated=30)	0	X
Significan	t stress relief fractures in abutments (no=0 or treated=3 or untreated=30)	0	Υ
History of	underground mining under embankment area (no=0 or treated=3 or untreated=30)	0	Z
Coarse g	rained and highly permeable soils (no=0 or yes=3)	0	AA
Presence	of weak layers/conditions diminishing embankment stability (no=0 or yes=3)	0	AB
Erodible s	soils (sandy/silty materials) or weakly cemented rock (no=0 or yes=3)	0	AC
Reservoir	area prone to landslides that could cause overtopping (no=0 or yes=3)	0	AD
EMBANKN	IENT DESIGN AND CONSTRUCTION (24 points max): (total points)		AE
Filters for	core or foundation or incompatibility between zones (no=3 or yes=0)	0	AF
Embankn	nent or foundation drainage system (yes=0 or no=4)	0	AG
Erodible (	core material (sands, silts, dispersive clays) (no=0 or yes=4)	0	AH
Incomplet	te or no foundation cutoff of shallow permeable layers (no=0 or yes=4)	0	Al
Poorly pla	aced earthfill, inadequate density (no=0 or yes=4)	0	AJ
	ures to drain reservoir (yes=0 or no=4)	0	AK
EMBANKN	IENT MONITORING (15 points max): (total points) 6		AL
	nts (except surficial survey points) installed at dam (yes=0 or no=3)	3	AM
Installed i	nstruments routinely read and evaluated (yes=0 or no=3)	3	AN
	pection of dam by engineer less often than yearly (no=0 or yes=3)	0	AO
	sical/visual access to downstream groin/toe for inspection (yes=0 or no=3)	0	AP
STATIC FA	ILURE INDEX: A+I+L+U+AE+AL	126	AQ

		- 1	EVALUATI	ON (	OF POTE	ITIAL REH	ABILITAT	ION PRO	JECTS	-	103
STATE	OH D	AM	Margaret (	Cree	k Str 6	1-5-VAL 3325.40		BY	SJJ	DATE	8/20/09
sht 4 of 5	SOUL EMPLOYEE		-21600 000-000	ŀ	IYDROLO	GIC FAILU	RE INDE	(		7. 10.341. 3	ver 10010
HYDROLO	GIC LOA	DING									
Total Spil	lway Cap	acity (	PS&ES) fo	r 6hr	storm [Pf	b], Work Pl	an Tbl 3 (ra	ainfall inc	hes)		13
Obtaine	ed from W	ork P	an Tbl 3, o	r dar	ns invento	ry data, or	computer r	outings			
100 year,	6hr rainfa	all [P1	00] (inches	)							4.25 E
Probable	Maximum	Pred	ipitation [Pl	MP]	(inches)						27.2
if Pfb <	P100			=	4.25	enter	40				
if Pfb =	P100+0	).2(PN	IP-P100)	=	8.84	enter	25				
if Pfb =	P100+0	).4(PN	IP-P100)	=	13.43	enter	15				
if Pfb =	P100+0	).6(PN	IP-P100)	=	18.02	enter	7				
if Pfb =	P100+0	).8(PN	IP-P100)	=	22.61	enter	3				
if Pfb =	PMP			=	27.2	enter	1				·
Ente	er interpol	lated v	value								15.9
HYDROLO	GIC UNC	ERTA	INTY:								<u></u> p
Drainage	Area [DA	] (squ	are miles)								4
DA<10	enter 1.5	; 10⊲	DA<20 ente	r 1.4	: 20 <da< td=""><td>&lt;50 enter 1</td><td>3 ; DA=&gt;5</td><td>0 enter 1</td><td>.2</td><td></td><td>1.5</td></da<>	<50 enter 1	3 ; DA=>5	0 enter 1	.2		1.5
PIPE SPILI	LWAY PL	UGG	NG:								
Pipe Dian	neter [D] (	(inche	5)								30
D<12 e	nter 1.1;	12<=[	0<24 enter	1.0;	24<=D en	ter 0.9					0.9 H
Riser & tr	ash rack t	type:									
Non-sta	andardized	d inlet	enter 1.1,	Oper	Top rise	enter 1.0;	Covered o	r Baffle T	op enter (	0.9	0.9
EARTH SP	ILLWAY	FLOV	V:								
Earth spil	lway flow	depth	[Des] from	top	of dam to	spillway cre	est (feet)(1	0' max)			8 .
DAM EROS	SION RES	SISTA	NCE:								
Non-plast	tic (PI<10)	) fill er	nter 2.0 ; PI	astic	core ente	r 1.7 ; Ove	topping ar	moring e	nter 0.8		1.7
Vegetal C	Cover Fac	tor [Ci	, see SITE	S or	AH667						0.87
http://w	ww.pswcr	l.ars.u	ısda.gov/al	1667	ah667.htr	n					500
Cf<0.4	enter 1.1	; Cf <	0.7 enter 1	.0; C	f<1.0 ente	er 0.9; large	r Cf enter	8.0			0.9
EARTH SP	ILLWAY	EROS	ION RESI	STA	NCE:						
Low, can	be excav	ated v	vith hand to	ols,	enter 2.0						
PI>10 a	and SPT b	lows<	8, PI<10 a	nd S	PT blows	8, Kh<0.10	, seismic v	velocity<2	0000fps		
Moderate	, can be e	excava	ated with co	onstr	uction equ	ipment, ea	sy ripping,	enter 1.2			
PI>10 a	and SPT b	lows>	8, PI<10 a	nd S	PT blows	30, Kh<10	seismic v	elocity<7	000fps		
High, ven	y hard rips	ping, r	equires dril	ling .	and blasti	ng, enter 0.	2				12.
modera	tely hard	rock,	Kh>10, seis	smic	velocity>7	000fps					1.2 N
Vegetal C	Cover Fac	tor [C	, see SITE	S or	AH667						0.87
Cf < 0.4	enter 1.1	; Cf <	0.7 enter 1	.0; C	f<1.0 ente	er 0.9; large	r Cf enter	8.0			0.9 F
HYDROLO	GIC FAIL	URE	INDEX:								<u></u>
dam over	topping b	reach	(2)(D)(F)	(H)(I	)(K)(M)						59
earth spil	lway brea	ch:	(D+5J)(F)(H	1)(1)(	N)(P)						73 F
larger of (	(2)(D)(F)(I	H)(I)(H	()(M) or (E	)+5J	)(F)(H)(I)(I	N)(P) but le	ess than 30	00			73 5

		<b>EVALUATION OF</b>	POTENTIAL REHA	BILITATION	PROJECTS		40
STATE	OH DAM	Margaret Creek	Str 6	BY	SJJ	DATE	8/20/09
sht 5 of 5			SEISMIC FAILURE	INDEX		1.11.20-1.31.	ver 102201
SEISMIC L	OADING:						
Latitude	e (degrees.ded	cimal)					39.3 A
Longitu	de (degrees.d	ecimal)					82.19 B
See "http	://eqint.cr.usgs	s.gov/eq/html/looku	ıp.shtml"				53 (7)
PGA [pea	ak ground acce	eleration] for 2% ch	nance in 50 years, se	e NEHRP map	os (%g)		5.43 C
ifPGA	is less than 10	% g, enter 0					90
ifPGA	is between 10°	% g and 19% g, en	nter 0.15				
if PGA	is between 20°	% g and 39% g, en	nter 0.30				
if PGA	is between 40°	% g and 59% g, en	iter 0.65				
ifPGA	is greater than	60% g, enter 1.0					0 D
FOUNDAT	ION LIQUEFA	CTION:					
Select on	ly one of the fo	ollowing foundation	conditions which be	est represents	the site		10 10
Loose all	uvium, lacustri	ine, loess materials	s (no=0 or yes=10)				0 E
Bedrock,	glacial till, high	hly clayey materials	s (no=0 or yes=5)				5 F
EMBANKN	MENT FREEBO	DARD FOR FOUN	DATION LIQUEFAC	TION:			(4)
Dam heig	ht for seismic	event is the height	t from top of dam to o	downstream ch	annel botton	n (ft)	42 G
Freeboar	d for seismic e	event is the depth f	rom top of dam to as	sumed pool su	ırface (ft)		8 H
Freebo	oard percent of	f dam height (%)					19 1
if Freebo	ard is less than	n 25% of dam heig	ht, enter 10				
if Freebo	ard is 25% to 5	50% of dam height,	, enter 5				
if Freebo	ard is more tha	an 50% of dam hei	ght, enter 1				10 J
EMBANKN	MENT FREEBO	DARD FOR EMBA	NKMENT CRACKIN	IG:			
Freeboar	d is less than o	or equal to 15 feet	(no=0 or yes=1)				1 K
EMBANKN	MENT CRACK	ING:					10
Embankn	nent contains	self-healing filter zo	ones (no=4 or yes=0)	)			4 L
SEISMIC F	AILURE INDE	X:	1170.000				
(D) ( (E)(	J) + (F)(K+1)(L	L+1)) but less than	n 100				0 M
	Mic	half	Mans	in			
		rvation Er gin er's					
lo .	concurring wi	th technical conten	nt of sheets 2 thru 5				